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A New Higher-Order Shear and Normal Deformation Theory for the Free Vibration Analysis of Laminated Shells

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KEYWORDS

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ABSTRACT

In this paper, the free vibration analysis of laminated composite and sandwich, cylindrical and spherical shells is presented using a new higher-order shear and normal deformation theory. The novelty of the present theory is that it includes the effects of both transverse shear and normal deformations along with higher order expansions of displacement field. A fifth-order polynomial type shape function is used in the in-plane displacements to represent the effect of transverse shear deformation for the first time whereas transverse displacement is a function of x , y , and z coordinates to account for the effect of transverse normal deformation. The equations of motion are derived using Hamilton's principle. Navier's solution technique is employed to obtain the non-dimensional fundamental frequencies. To validate the accuracy of the present theory, the present results are compared with other higher-order theories available in the literature. It is observed that the values of fundamental frequencies obtained using the present theory are in close agreement with those available in the literature.

1. Introduction

Laminated composite and sandwich shells are having a wide application in the area of aircraft, spacecraft, undermining, marine constructions, etc. due to their attractive features such as high load carrying capacity, large span-to-depth ratio, high strength-to-weight ratio, high stiffness-to-weight ratio, etc.

In the case of mechanical and structural industries, the structural components get subjected to extreme loads and deformations due to vibration and resonance, which leads to catastrophic failure. Also, in the case of aircraft, to avoid the severe consequences during in-flight conditions the wings need to be designed to eliminate the resonance, in the case of civil structures it needs to be designed considering the wind-induced vibration.

Therefore, static and vibration analysis of laminated composite shells becomes an active area of research among researchers. 169 years

ago Kirchoff [1] has developed a classical shell theory (CST) for the analysis of thin shells which neglect the effect of shear deformation. However, this theory is not useful for the analysis of thick shells. Therefore, Mindlin [2] has developed a first-order shear deformation theory (FSDT) which considers the effect of shear deformation for the first time. However, this theory fails to satisfy the zero transverse shear stress condition at top and bottom surfaces of the shell. These drawbacks of the CST and FSDT lead to the development of higher-order shear deformation theories (HSDT). Qatu [3-5], Qatu et al. [6], Mallikarjuna and Kant [7], Thai and Kim [8], Sayyad and Ghugal [9-11] have published several review articles on free vibration analysis of laminated composite beams, plates, and shells.

Reddy [12] has developed a well-known parabolic shear deformation theory (PSDT) for the analysis of laminated composite plates and shells. Bhimaraddi [13] has presented a free

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vibration analysis of doubly curved shells using three dimensional elasticity theory assuming that the ratio of shell thickness to its middle surface radius is negligible. Timaraci and Soldatos [14] studied the dynamic behavior of symmetric cross-ply circular shells using various shear deformation theories. Khare et al. [15] have developed a finite element formulation using a higher-order facet shell element for the free vibration analysis of laminated composite and sandwich cylindrical and spherical shells. A layerwise shear deformation theory is developed by Ferreira [16] for the static analysis of laminated composite plates. Static and free vibration analysis of laminated composite shells is presented by Ferreira et al. [17] by developing a meshless solution of Reddy's higher-order shell theory. Garg et al. [18] presented a closed-form solution for the free vibration analysis of doubly curved laminated composite and sandwich shells. Pradyumna and Bandopadhyay [19] investigated a C0 finite element formulation based on higher-order shear deformation theory for the static and free vibration analysis of laminated composite shells. Matsunga [20] presented free vibration and stability analysis of cross-ply laminated composite shells, considering the effect of transverse shear and normal deformations. Carrera and Brischetto [21] have presented an analysis of laminated composite shells using refined and mixed shear deformation theories. Brischetto et al. [22] studied a free vibration analysis of sandwich plates and shells by introducing a zig-zag function in the displacement field of classical and higher-order theories. Zhao et al. [23] have applied the Ritz method for the static and dynamic analysis of functionally graded cylindrical shells. Noh and Lee [24] have presented the free vibration analysis of laminated composite shells by developing a finite element formulation based on a third-order shear deformation theory. Bending and free vibration analysis of laminated composite plates and shells is presented by Mantari and Soares [25] using higher-order shear deformation theory. Tornabene [26, 27], Tornabene et al. [28, 29] have proposed a GDQ method for the free vibration analysis of laminated composite and functionally graded shells. Qatu and Asadi [30] presented a comprehensive study on free vibration analysis of spherical, cylindrical, and hyperbolic paraboloidal shells using a Ritz method for various boundary conditions. Asadi et al. [31] have presented a 3D solution for static and vibration analysis of thick deep laminated cylindrical shells. The hierarchical trigonometric Ritz formulation is used by Fazzolari and Carrera [32] for the free vibration analysis of doubly curved laminated composite shells. Taj and

Chakrabarti [33] have studied the bending analysis of functionally graded ceramic-metal skew shell panels using a C0 finite element. Dai et al. [34] have obtained an exact series solution for the free vibration analysis of cylindrical shells for various boundary conditions. Wang et al. [35] have predicted the free vibration response of laminated composite circular panels and shells of revolution using a Fourier-Ritz method for various boundary conditions. Rawat et al. [36] have developed a finite element model for the free vibration analysis of thin circular cylindrical shells. Pandey and Pradyumna [37] have presented a thermally induced vibration analysis of functionally graded sandwich plates and shell panels. Biswal et al. [38] studied free vibration and stability analysis of doubly curved laminated shell panels based on Sander's approximation. Fares et al. [39] have presented the bending and free vibration analysis of functionally graded doubly curved shells using a layerwise theory. Monge et al. [40] have carried out an asymptotic evaluation of the best theories for the free vibration analysis of laminated composite and sandwich shells. Cong et al. [41] have extended a third order shear deformation theory of Reddy by developing a new approach to investigate the nonlinear dynamic response of doubly curved sandwich shells. A semi-analytical method is used by Li et al. [42] and Pang et al. [43] to study the free vibration analysis for laminated composite doubly curved shells of revolution. Kiani et al. [44] investigated the free vibration characteristics of functionally graded carbon nanotube skew cylindrical shells based on Chebyshev-Ritz formulation. Sayyad and Ghugal [45] developed a generalized shell theory to investigate the static and dynamic response of laminated composite and sandwich spherical shells. Draiche et al. [46], Allam et al. [47], Zine et al. [48] presented a higher order shear deformation theory for the bending and free vibration analysis of laminated composite, sandwich plates, and shells. Arefi [49-53], Arefi and Rabczuk [54], Arefi and Elyas [55], Arefi and Amabili [56] have presented an electro-elastic and free vibration analysis of piezoelectric doubly curved nanoshells. Arefi and Zenkour [57-58] have presented the bending and free vibration analysis of functionally graded nanobeams whereas Arefi et al. [59] presented the bending response of FG composite doubly curved nanoshells considering the thickness stretching effects.

Draiche et al. [60] presented a static analysis of laminated reinforced plates using first order shear deformation theory. Belbachir et al. [61-62], Abualnour et al. [63], Sahla et al. [64] have applied a four variable refined theory for the static and free vibration analysis of laminated

composite plates and shells under mechanical and thermal load.

1.1. The Present Contribution

Carrera [65] reported in his research that for the accurate structural analysis of composite laminates, higher-order theories must be expanded up to minimum fifth-order polynomial. It is also recommended by Carrera that it is important to consider the effect of transverse normal deformation for the analysis of composite laminates. However, limited literature is available on refined theories considering the effects of transverse normal strain. Also, refined theories representing higher order (minimum fifth-order) expansion of displacement field are limited. Based on these observations Sayyad and his coauthors Sayyad and Naik, [66], Naik and Sayyad, [67-69], Ghumare and Sayyad [70-72], Shinde and Sayyad [73] have given due consideration to these recommendations of Carrera and developed a fifth-order shear and normal deformation theory for the analysis of laminated composite, sandwich, and functionally graded beams, plates, and shells. In the present work, this theory is extended for the free vibration analysis of laminated shells. The theory considers the effects of transverse shear and normal deformations which is neglected by classical theories as well as many other higher-order shell theories including Reddy's theory. A fifth-order polynomial type shear strain function is used in the in-plane displacements to consider the effect of transverse shear deformation, whereas a fourth-order function is considered in the transverse displacement to account for the effect of normal deformation i.e. transverse normal stress. The equations of motion are derived using Hamilton's principle. Navier's solution technique is employed to obtain the non-dimensional fundamental frequencies. The numerical results obtained for the natural frequencies using the present theory are compared with other higher-order theories presented by Asadi et al. [31], Bhimaraddi [13], Sayyad, and Ghugal [45], etc. In overall, the numerical results predicted by the present theory are in excellent agreement with the 3D elasticity solution.

2. Mathematical Formulation of the Present Theory

A simply supported laminated composite shell on rectangular planform with a width a in the x -direction, breadth b in the y -direction, and thickness h in the z -direction as shown in Fig. 1 is considered. The shell has N number of orthotropic layers made up of fibrous composite

materials. For spherical shell $R_1=R_2=R$ and for cylindrical shell $R_1=R, R_2=\infty$.

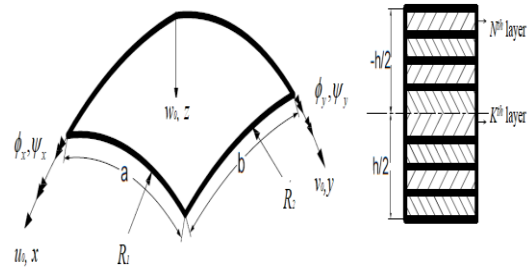


Fig. 1. Geometry and coordinate system of a spherical shell.

The present theory is built upon classical shell theory. Extension and bending components associated with the present theory are analogous to the classical shell theory. Fifth-order polynomial type shape functions are used in the in-plane displacements to account for the effect of transverse shear deformation. Whereas, fourth-order shape function in terms of thickness coordinates is used in the transverse displacement to account for the effect of transverse normal deformations. The theory is presented by Sayyad and Naik, [65], Naik and Sayyad, [67-69], Ghumare and Sayyad [70-72] and, Shinde and Sayyad [73]. The displacement field of the present theory is as follows.

$$\begin{aligned}
 u(x, y, z, t) &= \left(1 + \frac{z}{R_1}\right) u_0 - z \frac{\partial w_0}{\partial x} + \varphi_1(z) \phi_x + \varphi_2(z) \psi_x \\
 v(x, y, z, t) &= \left(1 + \frac{z}{R_2}\right) v_0 - z \frac{\partial w_0}{\partial y} + \varphi_1(z) \phi_y + \varphi_2(z) \psi_y \\
 w(x, y, z, t) &= w_0 + \varphi_1'(z) \phi_z + \varphi_2'(z) \psi_z
 \end{aligned}
 \tag{1}$$

where

$$\begin{aligned}
 \varphi_1(z) &= \left(z - \frac{4z^3}{3h^2}\right), \quad \varphi_2(z) = \left(z - \frac{16z^5}{5h^4}\right), \\
 \varphi_1'(z) &= \left(1 - \frac{4z^2}{h^2}\right), \quad \varphi_2'(z) = \left(1 - \frac{16z^4}{h^4}\right)
 \end{aligned}
 \tag{2}$$

where $u, v,$ and w are the displacements in the x, y - and z - directions, respectively; $u_0, v_0,$ and w_0 are the unknown displacements of the mid-plane of the shell in x, y - and z - directions respectively; (ϕ_x, ψ_x) are the shear slopes about the y -axis, (ϕ_y, ψ_y) are the shear slopes about the x -axis, (ϕ_z, ψ_z) are the shear slopes about the z -axis. All these shear slopes are unknowns that need to be determined. (') represents the derivative of the function with respect to the z coordinate. The present theory has nine unknowns. The following general strain-displacement relationships are used to determine nonzero strain components, Bhimaraddi [13].

$$\begin{Bmatrix} \varepsilon_x \\ \varepsilon_y \\ \varepsilon_z \\ \gamma_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{Bmatrix} = \begin{Bmatrix} \frac{\partial u}{\partial x} + \frac{w}{R_1} \\ \frac{\partial v}{\partial y} + \frac{w}{R_2} \\ \frac{\partial w}{\partial z} \\ \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \\ \frac{\partial w}{\partial x} + \frac{\partial u}{\partial z} - \frac{u_0}{R_1} \\ \frac{\partial w}{\partial y} + \frac{\partial v}{\partial z} - \frac{v_0}{R_2} \end{Bmatrix} \quad (3)$$

Substituting displacement expressions from the displacement field, stated in Eq. (1) one can obtain the following expressions for nonzero strains.

$$\begin{aligned} \varepsilon_x &= \left(\frac{\partial u_0}{\partial x} + \frac{w_0}{R_1} \right) - z \frac{\partial^2 w_0}{\partial x^2} + \varphi_1(z) \frac{\partial \phi_x}{\partial x} \\ &\quad + \varphi_2(z) \frac{\partial \psi_x}{\partial x} + \frac{\varphi_1'(z)}{R_1} \phi_z + \frac{\varphi_2'(z)}{R_1} \psi_z \\ \varepsilon_y &= \left(\frac{\partial v_0}{\partial y} + \frac{w_0}{R_2} \right) - z \frac{\partial^2 w_0}{\partial y^2} + \varphi_1(z) \frac{\partial \phi_y}{\partial y} \\ &\quad + \varphi_2(z) \frac{\partial \psi_y}{\partial y} + \frac{\varphi_1'(z)}{R_2} \phi_z + \frac{\varphi_2'(z)}{R_2} \psi_z \\ \varepsilon_z &= \varphi_1'(z) \phi_z + \varphi_2'(z) \psi_z \\ \gamma_{xy} &= \frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} - 2z \frac{\partial^2 w_0}{\partial x \partial y} + \varphi_1(z) \frac{\partial \phi_x}{\partial y} \\ &\quad + \varphi_1(z) \frac{\partial \phi_y}{\partial x} + \varphi_2(z) \frac{\partial \psi_x}{\partial y} + \varphi_2(z) \frac{\partial \psi_y}{\partial x} \\ \gamma_{xz} &= \varphi_1'(z) \phi_x + \varphi_2'(z) \psi_x + \varphi_1'(z) \frac{\partial \phi_z}{\partial x} \\ &\quad + \varphi_2'(z) \frac{\partial \psi_z}{\partial x} \\ \gamma_{yz} &= \varphi_1'(z) \phi_y + \varphi_2'(z) \psi_y + \varphi_1'(z) \frac{\partial \phi_z}{\partial y} \\ &\quad + \varphi_2'(z) \frac{\partial \psi_z}{\partial y} \end{aligned} \quad (4)$$

The three-dimensional Hooke's law is used to derive expressions for stresses in the shell domain.

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \tau_{xy} \\ \tau_{xz} \\ \tau_{yz} \end{Bmatrix}^k = [Q_{ij}]^k \begin{Bmatrix} \varepsilon_x \\ \varepsilon_y \\ \varepsilon_z \\ \gamma_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{Bmatrix} \quad (5)$$

where,

$$[Q_{ij}]^k = \begin{bmatrix} Q_{11} & Q_{12} & Q_{13} & 0 & 0 & 0 \\ Q_{12} & Q_{22} & Q_{23} & 0 & 0 & 0 \\ Q_{13} & Q_{23} & Q_{33} & 0 & 0 & 0 \\ 0 & 0 & 0 & Q_{66} & 0 & 0 \\ 0 & 0 & 0 & 0 & Q_{55} & 0 \\ 0 & 0 & 0 & 0 & 0 & Q_{44} \end{bmatrix} \quad (6)$$

$$\begin{aligned} Q_{11} &= \frac{E_1(1-\mu_{23}\mu_{32})}{\Delta}, Q_{12} = \frac{E_1(\mu_{21} + \mu_{23}\mu_{31})}{\Delta}, \\ Q_{13} &= \frac{E_1(\mu_{31} + \mu_{21}\mu_{32})}{\Delta}, Q_{22} = \frac{E_2(1-\mu_{13}\mu_{31})}{\Delta}, \\ Q_{23} &= \frac{E_2(\mu_{32} + \mu_{21}\mu_{31})}{\Delta}, Q_{33} = \frac{E_3(1-\mu_{12}\mu_{21})}{\Delta} \quad (7) \end{aligned}$$

$$Q_{66} = G_{12}, \quad Q_{55} = G_{13}, \quad Q_{44} = G_{23},$$

where

$$\Delta = 1 - \mu_{21}\mu_{12} - \mu_{23}\mu_{32} - \mu_{31}\mu_{13} - 2\mu_{12}\mu_{23}\mu_{31}$$

E_i 's ($i=1, 2, 3$) are the modulus of elasticity; G_{ij} 's are the modulus of rigidity, and μ_{ij} 's are the Poisson's ratio. The equations of motion for the free vibration analysis of the shell are obtained using Hamilton's principle.

$$\int_{t_1}^{t_2} [\delta U - \delta V + \delta K] dt = 0 \quad (8)$$

where, δK represents the kinetic energy due to inertia forces, δU represents the strain energy due to stresses, and δV represents the potential energy due to external load. δ is the variational operator. t_1 and t_2 are the initial and final times respectively. Substituting values of these energies in Eq. (8), one can rewrite the Eq. (8) as

$$\begin{aligned} &\int_{dv} \sigma_{ij} \delta \varepsilon_{ij} dv - \int_{dA} q(x, y) \delta w dA + \\ &\rho \int_{dv} \left(\frac{\partial^2 u}{\partial t^2} \delta u + \frac{\partial^2 v}{\partial t^2} \delta v + \frac{\partial^2 w}{\partial t^2} \delta w \right) dv = 0 \end{aligned} \quad (9)$$

where ρ represents the mass density. Substituting the expressions of stresses and strains from Eqs. (3)-(7) into the Eq. (9), integrating by parts, collecting the coefficients of $(\delta u_0, \delta v_0, \delta w_0, \delta \phi_x, \delta \phi_y, \delta \phi_z, \delta \psi_x, \delta \psi_y, \delta \psi_z)$ and setting them equal to zero, one can derive the following equations of motion.

$$\begin{aligned}
 \delta u_0 &: \frac{\partial N_x}{\partial x} + \frac{\partial N_{xy}}{\partial y} \\
 &= \left(I_1 + 2 \frac{I_2}{R_1} + \frac{I_3}{R_1^2} \right) \frac{\partial^2 u}{\partial t^2} - \left(I_2 + \frac{I_3}{R_1} \right) \frac{\partial^3 w}{\partial x \partial t^2} \\
 &+ \left(I_4 + \frac{I_8}{R_1} \right) \frac{\partial^2 \phi_x}{\partial t^2} + \left(I_5 + \frac{I_9}{R_1} \right) \frac{\partial^2 \psi_x}{\partial t^2} \\
 \delta v_0 &: \frac{\partial N_y}{\partial y} + \frac{\partial N_{xy}}{\partial x} \\
 &= \left(I_1 + 2 \frac{I_2}{R_2} + \frac{I_3}{R_2^2} \right) \frac{\partial^2 v}{\partial t^2} - \left(I_2 + \frac{I_3}{R_2} \right) \frac{\partial^3 w}{\partial y \partial t^2} \\
 &+ \left(I_4 + \frac{I_8}{R_2} \right) \frac{\partial^2 \phi_y}{\partial t^2} + \left(I_5 + \frac{I_9}{R_2} \right) \frac{\partial^2 \psi_y}{\partial t^2} \\
 \delta w_0 &: \frac{\partial^2 M_x^b}{\partial x^2} + \frac{\partial^2 M_y^b}{\partial y^2} + 2 \frac{\partial^2 M_{xy}^b}{\partial x \partial y} - \frac{N_x}{R_1} - \frac{N_y}{R_2} + q \\
 &= \left(I_2 + \frac{I_3}{R_1} \right) \frac{\partial^3 u}{\partial x \partial t^2} - I_3 \frac{\partial^4 w}{\partial x^2 \partial t^2} + I_8 \frac{\partial^3 \phi_x}{\partial x \partial t^2} \\
 &+ I_9 \frac{\partial^3 \psi_x}{\partial x \partial t^2} + \left(I_2 + \frac{I_3}{R_2} \right) \frac{\partial^3 v}{\partial y \partial t^2} - I_3 \frac{\partial^4 w}{\partial y^2 \partial t^2} + \\
 &I_8 \frac{\partial^3 \phi_y}{\partial y \partial t^2} + I_9 \frac{\partial^3 \psi_y}{\partial y \partial t^2} + I_{10} \frac{\partial^2 \phi_z}{\partial t^2} + I_{11} \frac{\partial^2 \psi_z}{\partial t^2} \\
 \delta \phi_x &: \frac{\partial M_x^{s1}}{\partial x} + \frac{\partial M_{xy}^{s1}}{\partial y} - Q_x^{s1} = \\
 &\left(I_4 + \frac{I_8}{R_1} \right) \frac{\partial^2 u}{\partial t^2} - I_8 \frac{\partial^3 w}{\partial x \partial t^2} + I_6 \frac{\partial^2 \phi_x}{\partial t^2} + I_{15} \frac{\partial^2 \psi_x}{\partial t^2} \\
 \delta \psi_x &: \frac{\partial M_x^{s2}}{\partial x} + \frac{\partial M_{xy}^{s2}}{\partial y} - Q_x^{s2} = \\
 &\left(I_5 + \frac{I_9}{R_1} \right) \frac{\partial^2 v}{\partial t^2} - I_9 \frac{\partial^3 w}{\partial x \partial t^2} + I_{15} \frac{\partial^2 \phi_x}{\partial t^2} + I_7 \frac{\partial^2 \psi_x}{\partial t^2} \\
 \delta \phi_y &: \frac{\partial M_y^{s1}}{\partial y} + \frac{\partial M_{xy}^{s1}}{\partial x} - Q_y^{s1} = \\
 &\left(I_4 + \frac{I_8}{R_2} \right) \frac{\partial^2 v}{\partial t^2} - I_8 \frac{\partial^3 w}{\partial y \partial t^2} + I_6 \frac{\partial^2 \phi_y}{\partial t^2} + I_{15} \frac{\partial^2 \psi_y}{\partial t^2} \\
 \delta \psi_y &: \frac{\partial M_y^{s2}}{\partial y} + \frac{\partial M_{xy}^{s2}}{\partial x} - Q_y^{s2} = \\
 &\left(I_5 + \frac{I_9}{R_2} \right) \frac{\partial^2 v}{\partial t^2} - I_9 \frac{\partial^3 w}{\partial y \partial t^2} + I_{15} \frac{\partial^2 \phi_y}{\partial t^2} + I_7 \frac{\partial^2 \psi_y}{\partial t^2} \\
 \delta \phi_z &: \frac{\partial Q_x^{s1}}{\partial x} + \frac{\partial Q_y^{s1}}{\partial y} - \frac{S_{xs1}}{R_1} - \frac{S_{ys1}}{R_2} - S^{s1} = \\
 &I_{10} \frac{\partial^3 w}{\partial t^2} + I_{13} \frac{\partial^2 \phi_z}{\partial t^2} + I_{12} \frac{\partial^2 \psi_z}{\partial t^2} \\
 \delta \psi_z &: \frac{\partial Q_x^{s2}}{\partial x} + \frac{\partial Q_y^{s2}}{\partial y} - \frac{S_{xs2}}{R_1} - \frac{S_{ys2}}{R_2} - S^{s2} = \\
 &I_{11} \frac{\partial^3 w}{\partial t^2} + I_{12} \frac{\partial^2 \phi_z}{\partial t^2} + I_{14} \frac{\partial^2 \psi_z}{\partial t^2}
 \end{aligned}
 \tag{10}$$

Boundary conditions of the present theory are expressed in the following form

Along the edges $x=0$ and $x=a$

Either $u_0 = 0$ or N_x is prescribed

Either $v_0 = 0$ or N_{xy} is prescribed

Either $w_0 = 0$ or $\frac{dM_x^b}{dx} + 2 \frac{dM_{xy}^b}{dy}$ is prescribed

Either $\frac{\partial w_0}{\partial x} = 0$ or M_{xy}^b is prescribed

Either $\phi_x = 0$ or M_x^{s1} is prescribed

Either $\psi_x = 0$ or M_x^{s2} is prescribed

Either $\phi_y = 0$ or M_{xy}^{s1} is prescribed

Either $\psi_y = 0$ or M_{xy}^{s2} is prescribed

Either $\phi_z = 0$ or Q_x^{s1} is prescribed

Either $\psi_z = 0$ or Q_{xz}^{s2} is prescribed

Along with the edges $y=0$ and $y=b$,

Either $u_0 = 0$ or N_{xy} is prescribed

Either $v_0 = 0$ or N_y is prescribed

Either $w_0 = 0$ or $\frac{dM_y^b}{dy} + 2 \frac{dM_{xy}^b}{dx}$ is prescribed

Either $\frac{\partial w_0}{\partial y} = 0$ or M_{xy}^b is prescribed

Either $\phi_x = 0$ or M_{xy}^{s1} is prescribed

Either $\psi_x = 0$ or M_{xy}^{s2} is prescribed

Either $\phi_y = 0$ or M_y^{s1} is prescribed

Either $\psi_y = 0$ or M_y^{s2} is prescribed

Either $\phi_z = 0$ or Q_y^{s1} is prescribed

Either $\psi_z = 0$ or Q_{yz}^{s2} is prescribed

where force and moment resultants can be derived from the following relations

$$\begin{aligned}
 (N_x, N_y, N_{xy}) &= \int_{-h/2}^{h/2} [\sigma_x, \sigma_y, \tau_{xy}] dz \\
 (M_x^b, M_y^b, M_{xy}^b) &= \int_{-h/2}^{h/2} [z\sigma_x, z\sigma_y, z\tau_{xy}] dz \\
 (M_x^{s1}, M_y^{s1}, M_{xy}^{s1}) &= \int_{-h/2}^{h/2} \left\{ [\phi_1(z)(\sigma_x, \sigma_y, \tau_{xy})] \right\} dz \\
 (M_x^{s2}, M_y^{s2}, M_{xy}^{s2}) &= \int_{-h/2}^{h/2} \left\{ [\phi_2(z)(\sigma_x, \sigma_y, \tau_{xy})] \right\} dz \\
 (Q_x^{s1}, Q_y^{s1}) &= \int_{-h/2}^{h/2} \left\{ [\phi_1(z)(\tau_{xz}, \tau_{yz})] \right\} dz \\
 (Q_x^{s2}, Q_y^{s2}) &= \int_{-h/2}^{h/2} \left\{ [\phi_2(z)(\tau_{xz}, \tau_{yz})] \right\} dz \\
 (S^{s1}, S^{s2}) &= \int_{-h/2}^{h/2} \left\{ \sigma_z [\phi_1(z), \phi_2(z)] \right\} dz \\
 (S_{xs1}, S_{xs2}) &= \int_{-h/2}^{h/2} \left\{ \sigma_x [\phi_1(z), \phi_2(z)] \right\} dz \\
 (S_{ys1}, S_{ys2}) &= \int_{-h/2}^{h/2} \left\{ \sigma_y [\phi_1(z), \phi_2(z)] \right\} dz
 \end{aligned}
 \tag{11}$$

The inertia constants are derived as,

$$\begin{aligned}
 (I_1, I_2, I_3, I_4, I_5) &= \rho \int_{-h/2}^{h/2} [1, z, z^2, \varphi_1(z), \varphi_2(z)] dz \\
 (I_6, I_8, I_{15}) &= \rho \int_{-h/2}^{h/2} \varphi_1(z) [\varphi_1(z), z, \varphi_2(z)] dz \\
 (I_7, I_9) &= \rho \int_{-h/2}^{h/2} \varphi_2(z) [\varphi_2(z), z] dz \\
 (I_{10}, I_{12}, I_{13}) &= \rho \int_{-h/2}^{h/2} \varphi_1'(z) [1, \varphi_2'(z), \varphi_1'(z)] dz \\
 (I_{11}, I_{14}) &= \rho \int_{-h/2}^{h/2} \varphi_2'(z) [1, \varphi_2'(z)] dz
 \end{aligned} \tag{12}$$

3. Analytical Solutions

The Navier’s solution technique is used to obtain the analytical solutions for the free vibration analysis of simply supported cross-ply laminated composite shells. The boundary conditions at the simply supported edges of the shell are as follows.

Along the edges $x=0$ and $x=a$

$$v_0 = w_0 = \phi_y = \psi_y = \phi_z = \psi_z = 0 \tag{13}$$

$$M_x^b = M_x^{s1} = M_x^{s2} = N_x = 0$$

Along the edges $y=0$ and $y=b$

$$u_0 = w_0 = \phi_x = \psi_x = \phi_z = \psi_z = 0 \tag{14}$$

$$M_y^b = M_y^{s1} = M_y^{s2} = N_y = 0$$

The following trigonometric form of unknown variables is assumed which satisfies the simply supported boundary conditions exactly.

$$\begin{bmatrix} u_0 \\ \phi_x \\ \psi_x \\ v_0 \\ \phi_y \\ \psi_y \\ w_0 \\ \phi_z \\ \psi_z \end{bmatrix} = \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \begin{bmatrix} u_1 \cos \alpha x \sin \beta y e^{i\omega t} \\ \phi_{x1} \cos \alpha x \sin \beta y e^{i\omega t} \\ \psi_{x1} \cos \alpha x \sin \beta y e^{i\omega t} \\ v_1 \sin \alpha x \cos \beta y e^{i\omega t} \\ \phi_{y1} \sin \alpha x \cos \beta y e^{i\omega t} \\ \psi_{y1} \sin \alpha x \cos \beta y e^{i\omega t} \\ w_1 \sin \alpha x \sin \beta y e^{i\omega t} \\ \phi_{z1} \sin \alpha x \sin \beta y e^{i\omega t} \\ \psi_{z1} \sin \alpha x \sin \beta y e^{i\omega t} \end{bmatrix} \tag{15}$$

where $\alpha = m\pi/a$, $\beta = n\pi/b$; $i = \sqrt{-1}$; ω is the fundamental frequency and $u_1, \phi_{x1}, \psi_{x1}, v_1, \phi_{y1}, \psi_{y1}, w_1, \phi_{z1}, \psi_{z1}$ are the unknown parameters to be determined. Substitution of Eq. (15) into Eq. (10) by setting $q = 0$ leads to the following eigenvalue problem.

$$\{[K] - \omega^2 [M]\} \{\Delta\} = \{0\} \tag{16}$$

where $[K]$ is stiffness matrix, $[M]$ is the mass matrix, and $\{\Delta\}$ is the vector of amplitudes.

Elements of these matrices are mentioned in Appendix.

4. Numerical Results and Discussion

In the present study, natural frequencies for a homogeneous two-layer ($0^0/90^0$) and three-layer ($0^0/90^0/0^0$) laminated composite cylindrical and spherical shells are obtained using MATLAB 2015a. In laminated shells, all layers are considered of equal thickness. The numerical results are obtained for different values of the R/a ratio and aspect (a/h) ratio. The following material properties for the laminated composite shells are used, Bhimaraddi [13]

$$\frac{E_1}{E_2} = 25, \frac{E_3}{E_2} = 1, \frac{G_{12}}{E_2} = \frac{G_{13}}{E_2} = 0.5, \frac{G_{23}}{E_2} = 0.2, \tag{17}$$

$$\mu_{12} = \mu_{13} = \mu_{23} = 0.25, \rho = \text{constant}$$

The natural frequency is presented in the following non-dimensional form unless and until specified,

$$\bar{\omega} = \omega(a^2/h) \sqrt{\rho/E_2} \tag{18}$$

The comparison of the first five natural frequencies of two-layer ($0^0/90^0$) and three-layer ($0^0/90^0/0^0$) laminated composite cylindrical shells is shown in Tables 1-2 respectively. Natural frequencies are presented for $a/h=10$ and 20 at $R/a=2, 1, 0.5$. Material properties of laminated shells are mentioned in Eq. (17). The comparison shows that the present results are in good agreement with those results presented by Asadi et al. [31] by different models. It is observed that as the a/h ratio and R/a ratio increases the natural frequency increases. It is found that the natural frequencies are decreasing with respect to the increase in radius of curvature (R/a).

Also, from Tables 1 and 2 it is clearly observed that the natural frequency is less in deep shells and more in shallow shells. The variation of fundamental frequency with respect to the aspect ratio (a/h) is also shown in Tables 1 and 2, which predicts that the frequency is found more in thick shells.

Table 3 shows a comparison of natural frequencies for varying modes of vibration of a two-layer $0^0/90^0$ laminated cylindrical shells at $a/h=10$ and $R/a=1$. The numerical results are obtained for $m=n=1, 2, 3, 4, 5, 6$ to change modes of vibration. The comparison shows that the present theory predicts natural frequencies in good agreement with those presented by Bhimaraddi [13] using various theories.

Table 4 shows the effect of R/a and h/a ratios on the natural frequencies of orthotropic and laminated composite cylindrical shells. Variations of natural frequencies in laminated composite cylindrical shells are presented with the help of Fig. 2.

Table 5 through 7 and Figures.3 through 6 deal with the natural frequencies of orthotropic and laminated composite spherical shells using the present theory. The present results are obtained for different modes of vibrations, a/h ratios, and R/a ratios. The comparison of all tables shows that the present results are compared with those presented by Bhimaraddi [13] and, Sayyad and Ghugal [45] using various theories. The present results are found in good agreement with previously published results for parameters.

From Tables 5 through 7 it is seen that the fundamental frequency of laminated shells decreases with an increase in the radii of curvature (R/a ratio). These results are also plotted in Figures 3 through 6.

Figures 5 and 6 show the variation of fundamental frequency with respect to modes of vibration (m, n) in a single layer (0^0) and a double-layer ($0^0/90^0$) laminated shell.

From Figures 5 and 6 it is observed that as the mode (m, n) increases the value of frequency also increases.

Table 8 shows the fundamental frequencies of three layered ($0^0/core/0^0$) sandwich spherical shells. The thickness of each face sheet is $0.1h$ and

the middle core is $0.8h$, where h is the total thickness of the shell. Following are the material properties used for the middle core of sandwich shell, Sayyad and Ghugal [46]:

$$\frac{Q_{12}}{Q_{11}} = 0.23319, \quad \frac{Q_{13}}{Q_{11}} = 0.010776,$$

$$\frac{Q_{22}}{Q_{11}} = 0.543104, \quad \frac{Q_{23}}{Q_{11}} = 0.098276,$$

$$\frac{Q_{33}}{Q_{11}} = 0.530172, \quad \frac{Q_{44}}{Q_{11}} = 0.266810,$$

$$\frac{Q_{55}}{Q_{11}} = 0.159914, \quad \frac{Q_{22}}{Q_{11}} = 0.262931$$

The elastic properties of the face sheets are assumed as ' C ' times the elastic properties of the core and the value of C are taken as 1, 2, 5, 10, and 15. From Table 8 it is observed that the result obtained by using the present theory are in good agreement with other theories available in the literature. The fundamental frequency of sandwich shells decreases with an increase in the radii of curvature (R/a).

Also, the results quoted in Table 8 show that the values of fundamental frequency increase with an increase in softness of the core.

Table 1. First five non-dimensional natural frequencies in two-layer ($0^0/90^0$) laminated composite cylindrical shell for varying a/R and a/h ratio ($R_1=R$ and $R_2=\infty$).

a/h	R/a	Theory	ω_1	ω_2	ω_3	ω_4	ω_5
20	2.0	Present	11.579	25.514	28.015	36.485	50.747
		FSDTQ [31]	11.530	25.357	27.913	36.324	50.210
		3D-FEM [31]	11.537	25.378	27.951	36.434	50.253
	1.0	Present	15.967	26.053	34.840	38.115	50.925
		FSDTQ [31]	15.859	25.648	34.867	37.831	50.263
		3D-FEM [31]	15.861	25.658	34.890	37.942	50.297
	0.5	Present	24.855	27.686	43.444	50.689	51.091
		FSDTQ [31]	24.809	26.193	42.664	49.382	51.170
		3D-FEM [31]	24.805	26.162	42.743	49.359	51.167
	2.0	Present	9.5271	21.995	22.464	30.415	39.651
		FSDTQ [31]	9.4577	21.676	22.150	29.959	38.608
		3D-FEM [31]	9.4855	21.743	22.246	30.193	38.745
10	1.0	Present	10.859	22.190	24.319	30.899	39.820
		FSDTQ [31]	10.666	21.705	24.090	30.368	38.722
		3D-FEM [31]	10.686	21.767	24.191	30.614	38.896
	0.5	Present	14.099	22.496	29.446	32.266	39.788
		FSDTQ [31]	13.771	21.037	29.574	31.200	38.073
		3D-FEM [31]	13.772	21.040	29.639	31.411	38.266

Table 2. First five non-dimensional natural frequencies in three-layer (0°/90°/0°) laminated composite cylindrical shell for varying a/R and a/h ratio ($R_1=R$ and $R_2=\infty$).

a/h	R/a	Theory	ω_1	ω_2	ω_3	ω_4	ω_5
20	2.0	Present	15.353	24.850	41.093	42.808	46.932
		FSDTQ [31]	15.551	21.646	37.022	46.309	48.938
		3D-FEM [31]	15.245	21.370	36.803	43.529	46.148
	1.0	Present	18.596	32.641	42.761	47.928	50.801
		FSDTQ [31]	18.710	21.974	36.794	49.770	49.852
		3D-FEM [31]	18.471	21.703	36.567	47.074	47.416
	0.5	Present	25.896	42.591	49.308	51.416	66.365
		FSDTQ [31]	23.178	25.978	35.923	52.746	57.077
		3D-FEM [31]	22.924	25.840	35.668	50.360	56.448
10	2.0	Present	11.982	19.324	28.921	31.916	32.717
		FSDTQ [31]	12.443	18.677	30.839	31.323	34.456
		3D-FEM [31]	11.769	18.159	28.600	30.471	31.928
	1.0	Present	12.854	21.665	28.786	32.931	34.688
		FSDTQ [31]	13.187	18.524	30.564	32.232	34.523
		3D-FEM [31]	12.590	18.005	29.732	30.189	32.037
	0.5	Present	15.134	27.579	28.281	33.701	42.195
		FSDTQ [31]	15.250	17.989	29.491	34.795	34.913
		3D-FEM [31]	14.840	17.468	29.094	32.464	33.046

Table 3. Non-dimensional natural frequencies in two-layer (0°/90°) laminated composite cylindrical shell for varying modes of vibration ($R_1=R, R_2=\infty, R/a=1$ and $a/h=10$). ($\bar{\omega} = \omega\sqrt{\rho/E_2}$)

n	Source	$m=1$	$m=2$	$m=3$	$m=4$	$m=5$	$m=6$
1	Present	1.0859	2.2190	3.9820	5.8774	7.7433	9.4882
	3D-Elasticity [13]	1.0408	2.4127	4.1157	5.9337	7.7818	9.6281
2	Present	2.4319	3.0899	4.554	6.3011	8.0866	9.7855
	3D-Elasticity [13]	2.0956	3.0069	4.4760	6.1778	7.9611	9.7672
3	Present	4.1705	4.5649	5.6724	7.1589	8.7769	10.367
	3D-Elasticity [13]	3.7949	4.4010	5.5338	6.9958	8.6193	10.316
4	Present	6.0501	6.3079	7.1502	8.3748	9.7868	11.225
	3D-Elasticity [13]	5.6331	6.0816	6.9643	8.1881	9.6232	11.177
5	Present	7.9595	8.1370	8.8001	9.8109	11.025	12.300
	3D-Elasticity [13]	7.4876	7.8550	8.5704	9.6035	10.864	12.274
6	Present	9.8524	9.9762	10.514	11.360	12.402	13.520
	3D-Elasticity [13]	8.6842	9.4979	10.254	11.143	12.258	13.536

Table 4. Non-dimensional natural frequencies in laminated composite cylindrical shell ($R_1=R, R_2=\infty$) for different R/a and a/h ratios. ($\bar{\omega} = \omega\sqrt{\rho/E_2}$)

R/a	Source	Orthotropic			0°/90°		
		$h/a=0.05$	$h/a=0.1$	$h/a=0.15$	$h/a=0.05$	$h/a=0.1$	$h/a=0.15$
1	Present	0.8727	1.2919	1.5740	0.7983	1.0859	1.3714
	3D-Elasticity [13]	0.8917	1.3241	1.6169	0.7868	1.0408	1.2909
2	Present	0.7602	1.2495	1.5609	0.5789	0.9527	1.2815
	3D-Elasticity [13]	0.7663	1.2674	1.5924	0.5725	0.9362	1.2537
3	Present	0.7354	1.2407	1.5582	0.5243	0.9231	1.2609
	3D-Elasticity [13]	0.7396	1.2562	1.5878	0.5207	0.9144	1.2450
4	Present	0.7263	1.2376	1.5573	0.5034	0.9120	1.2529
	3D-Elasticity [13]	0.7304	1.2522	1.5452	0.5011	0.9061	1.2409
5	Present	0.7220	1.2361	1.5568	0.4933	0.9067	1.2487
	3D-Elasticity [13]	0.7255	1.2503	1.5842	0.4916	0.9020	1.2384
10	Present	0.7163	1.2342	1.5563	0.4793	0.8989	1.2423
	3D-Elasticity [13]	0.7194	1.2473	1.5825	0.4785	0.8956	1.2337
20	Present	0.7149	1.2337	1.5562	0.4757	0.8965	1.2399
	3D-Elasticity [13]	0.7179	1.2463	1.5821	0.4750	0.8934	1.2314
∞	Present	0.7144	1.2336	1.5562	0.4743	0.8952	1.2381
	3D-Elasticity [13]	0.7173	1.2461	1.5812	0.4736	0.8917	1.2290

Table 5. Non-dimensional natural frequencies in two-layer (0°/90°) laminated composite spherical shell ($R_1=R_2=R$) for varying modes of vibration ($R/a=1$ and $a/h=10$). ($\bar{\omega} = \omega\sqrt{\rho/E_2}$)

n	Source	$m=1$	$m=2$	$m=3$	$m=4$	$m=5$	$m=6$
1	Present	1.4818	2.6042	4.3235	6.2130	8.0999	9.8893
	3D-Elasticity [13]	1.3997	2.4387	4.0531	5.8455	7.6895	8.7973
2	Present	2.5333	3.2067	4.6702	6.4332	8.2533	10.006
	3D-Elasticity [13]	2.4420	3.0452	4.4168	5.7938	8.0320	9.6841
3	Present	4.1621	4.5650	5.6853	7.2038	8.8718	10.534
	3D-Elasticity [13]	4.0841	4.4327	5.4741	6.9051	8.5234	10.226
4	Present	5.9659	6.2301	7.0901	8.3601	9.8434	11.383
	3D-Elasticity [13]	5.7102	6.1128	6.9091	8.0983	9.5244	11.083
5	Present	7.7957	7.9834	8.6702	9.7402	11.051	12.465
	3D-Elasticity [13]	7.4002	7.8904	8.5227	9.5188	10.766	12.177
6	Present	9.5939	9.7358	10.306	11.226	12.393	13.692
	3D-Elasticity [13]	9.0324	9.6948	10.214	11.065	12.164	13.440

Table 6. Non-dimensional natural frequencies in two-layer (0°/90°) laminated composite spherical shell ($R_1=R_2=R$) for different R/a and a/h ratios.

R/a	Theory	a/h				
		5	10	20	50	100
5	Present	7.6370	9.3431	10.932	16.7371	29.0279
	PSDT [45]	7.6781	9.3424	10.923	16.7059	29.0271
	ESDT [45]	7.7826	9.3759	10.931	16.7068	29.0272
10	Present	7.5733	9.0750	9.8931	11.8618	16.8058
	PSDT [45]	7.6122	9.0738	9.8893	11.8560	16.8218
	FSDT [45]	7.6482	9.0991	9.8978	11.8575	16.8222
50	Present	7.5527	8.9870	9.5339	9.7865	10.0960
	PSDT [45]	7.5908	8.9856	9.5323	9.7943	10.1312
	ESDT [45]	7.6974	9.0208	9.5420	9.7959	10.1316
100	Present	7.5520	8.9842	9.5225	9.7144	9.8121
	PSDT [45]	7.5902	8.9828	9.5209	9.7227	9.8487
	ESDT [45]	7.6967	9.0180	9.5307	9.7243	9.8491
Plate	Present	7.5753	9.0123	9.5498	9.7220	9.7476
	PSDT [45]	7.5899	9.9819	9.5171	9.6988	9.7527
	ESDT [45]	7.6965	9.0171	9.5269	9.7004	9.7531

Table 7. Non-dimensional natural frequencies in three-layer (0°/90°/0°) laminated composite spherical shell ($R_1=R_2=R$) for different R/a and a/h ratios.

R/a	Theory	a/h				
		5	10	20	50	100
5	Present	8.3515	12.0792	15.1567	20.4682	31.4974
	PSDT [45]	8.3200	12.0613	15.0499	20.2525	31.2192
	ESDT [45]	8.3425	12.0412	15.0365	20.2601	31.2189
10	Present	8.2908	11.8770	14.4306	16.6907	20.6521
	PSDT [45]	8.2593	11.8633	14.3366	16.5276	20.4844
	ESDT [45]	8.2820	11.8428	14.3225	16.5247	20.4837
50	Present	8.2711	11.8111	14.1887	15.2760	15.6296
	PSDT [45]	8.2396	11.7988	14.0991	15.1334	15.5166
	ESDT [45]	8.2625	11.7781	14.0847	15.1302	15.5158
100	Present	8.2705	11.8090	14.1811	15.2296	15.4460
	PSDT [45]	8.2390	11.7968	14.0916	15.0876	15.3352
	ESDT [45]	8.2619	11.7760	14.0772	15.0845	15.3343
Plate	Present	8.2878	11.8281	14.1997	15.2359	15.4063
	PSDT [45]	8.2388	11.7961	14.0891	15.0724	15.2742
	ESDT [45]	8.2617	11.7754	14.0747	15.0692	15.2734

Table 8. Non-dimensional natural frequencies in three-layer ($0^\circ/\text{core}/0^\circ$) sandwich laminated composite spherical shell ($R_1=R_2=R$) for different R/a ratios.

R/a	Model	C				
		1	2	5	10	15
5	Present	5.0011	5.9644	8.0349	10.2930	11.868
	PSDT [45]	5.0209	5.9690	8.0090	10.023	10.249
	ESDT [45]	5.0205	5.9683	8.0076	10.0216	10.246
10	Present	5.0480	5.9841	8.0227	10.041	10.269
	PSDT [45]	4.8082	5.7718	7.8223	10.035	11.563
	ESDT [45]	4.8274	5.6883	7.6248	9.9028	11.332
20	Present	4.8280	5.6882	7.6236	9.9008	11.329
	PSDT [45]	4.8556	5.7042	7.6392	9.9209	11.353
	ESDT [45]	4.7585	5.7225	7.7681	9.9693	11.485
50	Present	4.7771	5.6150	7.5237	9.8689	11.645
	PSDT [45]	4.7783	5.6156	7.5241	9.8692	11.645
	ESDT [45]	4.8061	5.6318	7.5399	9.8895	11.669
100	Present	4.7635	5.5953	7.4971	9.8623	11.741
	PSDT [45]	4.7630	5.5944	7.4956	9.8600	11.738
	ESDT [45]	4.7642	5.5951	7.4959	9.8602	11.738
Plate	Present	4.7425	5.7066	7.7507	9.9482	11.460
	PSDT [45]	4.7615	5.5923	7.4931	9.8610	11.754
	ESDT [45]	4.7610	5.5915	7.4916	9.8587	11.751

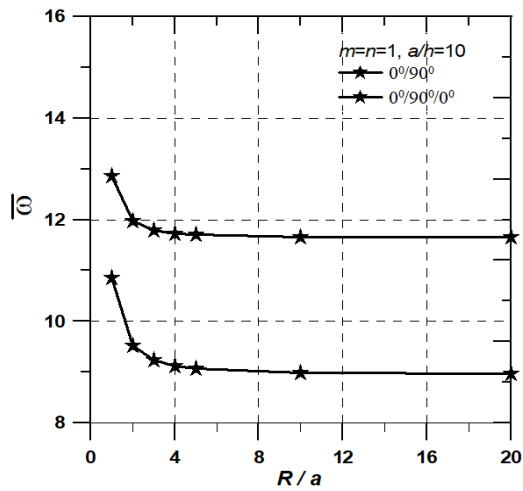


Fig. 2. Variations of natural frequencies with respect to R/a ratio in laminated composite cylindrical shell.

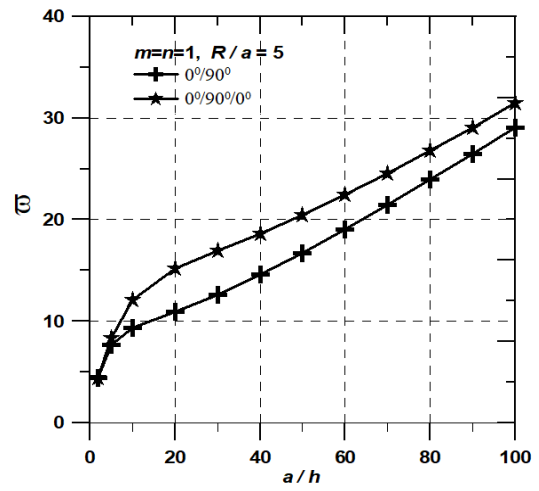


Fig. 4. Variations of natural frequencies with respect to a/h ratio in laminated composite spherical shell.

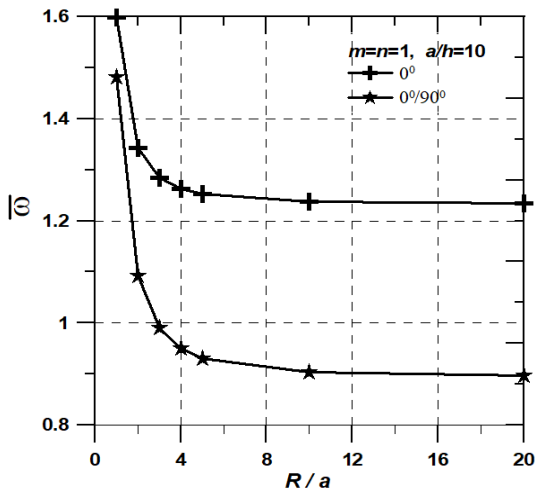


Fig. 3. Variations of natural frequencies with respect to R/a ratio in laminated composite spherical shell

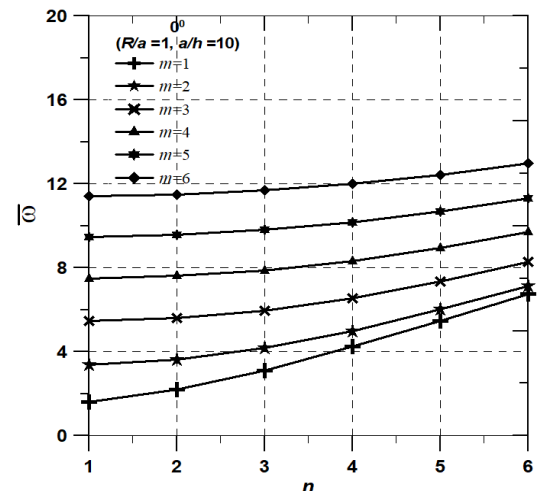


Fig. 5. Variations of natural frequencies with respect to modes of vibration in orthotropic spherical shell.

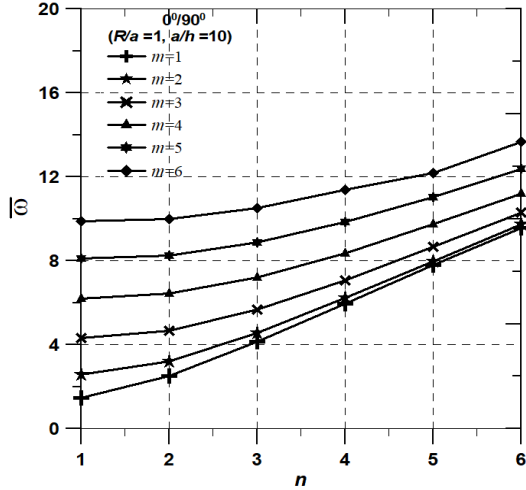


Fig. 6. Variations of natural frequencies ($\bar{\omega}$) with respect to modes of vibration in two layered ($0^\circ/90^\circ$) laminated composite spherical shell.

5. Conclusion

In the present study, a new fifth-order shear and normal deformation theory is developed and applied for the free vibration analysis of laminated composite and sandwich shells. The present theory includes the effects of both transverse shear and normal deformations. A polynomial type transverse shear strain shape function is used in the displacement field to account for these effects. The fundamental frequency analysis is performed for different types of shell problems, to prove the efficacy and validity of the present theory. Based on the numerical results and discussion, the following conclusions are drawn.

1. The present results are compared with previously published results and found in good agreement with those.
2. Natural frequencies of laminated shells decrease with an increase in the radii of curvature which shows that the deep shells predict higher frequencies whereas shallow shells predict lower frequencies.
3. It is concluded that the natural frequency increases with an increase in a/h ratio which ultimately shows that the thin shell predicts higher frequency whereas the thick shell predicts lower frequency.
4. In the case of sandwich shells, natural frequency increases with the increase in softness of the core material.

Based on the literature review, illustrated examples, numerical results, and discussion, the authors recommend the use of the present theory for many other problems of composite shells.

Appendix

The elements of stiffness matrix [K] in Eq. (16) are: [A.1]

$$\begin{aligned}
 K_{11} &= -A_{11}\alpha^2 - A_{66}\beta^2, & K_{12} &= K_{21} = -A_{12}\alpha\beta - A_{66}\alpha\beta, \\
 K_{13} &= K_{31} = \frac{A_{11}}{R_1}\alpha + \frac{A_{12}}{R_R}\beta + B_{11}\alpha^3 + B_{12}\alpha\beta^2 + 2B_{66}\alpha\beta^2, \\
 K_{14} &= K_{41} = -A_{S11}\alpha^2 - A_{S266}\beta^2, \\
 K_{15} &= K_{51} = -A_{S211}\alpha^2 - A_{S266}\beta^2, \\
 K_{16} &= K_{61} = -A_{S12}\alpha\beta - A_{S166}\alpha\beta, \\
 K_{17} &= K_{71} = -A_{S212}\alpha\beta - A_{S266}\alpha\beta, \\
 K_{18} &= K_{81} = \left(E_{13} + \frac{Q_{111}}{R_1} + \frac{Q_{112}}{R_2} \right) \alpha, \\
 K_{19} &= K_{91} = \left(F_{13} + \frac{Q_{211}}{R_1} + \frac{Q_{212}}{R_2} \right) \alpha, \\
 K_{22} &= -A_{22}\beta^2 - A_{66}\alpha^2, \\
 K_{23} &= K_{32} = \left(\frac{A_{12}}{R_1} + \frac{A_{22}}{R_2} \right) \beta + B_{12}\alpha^2\beta + B_{22}\beta^3 + 2B_{66}\alpha^2\beta, \\
 K_{24} &= K_{42} = -A_{S12}\alpha\beta - A_{S166}\alpha\beta, \\
 K_{25} &= K_{52} = -A_{S212}\alpha\beta - A_{S266}\alpha\beta, \\
 K_{26} &= K_{62} = -A_{S122}\beta^2 - A_{S166}\alpha^2, \\
 K_{27} &= K_{72} = -A_{S222}\beta^2 - A_{S266}\alpha^2, \\
 K_{28} &= K_{82} = \left(E_{23} + \frac{Q_{112}}{R_1} + \frac{Q_{122}}{R_2} \right) \beta, \\
 K_{29} &= K_{92} = \left(F_{23} + \frac{Q_{212}}{R_1} + \frac{Q_{222}}{R_2} \right) \beta \\
 K_{33} &= -(D_{11}\alpha^4 + D_{22}\beta^4) - 2\alpha^2\beta^2(D_{12} + 2D_{66}) \\
 &\quad - 2\alpha^2\left(\frac{B_{11}}{R_1} + \frac{B_{12}}{R_2}\right) - 2\beta^2\left(\frac{B_{12}}{R_1} + \frac{B_{22}}{R_2}\right) - \left(\frac{A_{11}}{R_1^2} + \frac{A_{22}}{R_2^2} + \frac{2A_{12}}{R_1R_2}\right), \\
 K_{34} &= K_{43} = B_{S11}\alpha^3 + B_{S12}\alpha\beta^2 + 2B_{S166}\alpha\beta^2 + \frac{A_{S111}}{R_1}\alpha + \frac{A_{S112}}{R_2}\alpha, \\
 K_{35} &= K_{53} = B_{S211}\alpha^3 + B_{S212}\alpha\beta^2 + 2B_{S266}\alpha\beta^2 + \frac{A_{S211}}{R_1}\alpha + \frac{A_{S212}}{R_2}\alpha, \\
 K_{36} &= K_{63} = B_{S12}\alpha^2\beta + B_{S122}\beta^3 + 2B_{S166}\alpha^2\beta + \frac{A_{S112}}{R_1}\beta + \frac{A_{S122}}{R_2}\beta, \\
 K_{37} &= K_{73} = B_{S212}\alpha^2\beta + B_{S222}\beta^3 + 2B_{S266}\alpha^2\beta + \frac{A_{S212}}{R_1}\beta + \frac{A_{S222}}{R_2}\beta, \\
 K_{38} &= K_{83} = -J_{13}\alpha^2 - J_{23}\beta^2 - \frac{E_{13}}{R_1} - \frac{E_{23}}{R_2} - \left(\frac{Q_{311}}{R_1} + \frac{Q_{312}}{R_2} \right) \alpha^2 \\
 &\quad - \left(\frac{Q_{312}}{R_1} + \frac{Q_{322}}{R_2} \right) \beta^2 - \left(\frac{Q_{111}}{R_1^2} + \frac{Q_{112}}{R_1R_2} \right) - \left(\frac{Q_{112}}{R_1R_2} + \frac{Q_{122}}{R_2^2} \right), \\
 K_{39} &= K_{93} = -O_{13}\alpha^2 - O_{23}\beta^2 - \frac{F_{13}}{R_1} - \frac{F_{23}}{R_2} - \left(\frac{Q_{411}}{R_1} + \frac{Q_{412}}{R_2} \right) \alpha^2 \\
 &\quad - \left(\frac{Q_{412}}{R_1} + \frac{Q_{422}}{R_2} \right) \beta^2 - \left(\frac{Q_{211}}{R_1^2} + \frac{Q_{212}}{R_1R_2} \right) - \left(\frac{Q_{212}}{R_1R_2} + \frac{Q_{222}}{R_2^2} \right),
 \end{aligned}$$

$$\begin{aligned}
 K_{44} &= -A_{SS11} \alpha^2 - A_{SS166} \beta^2 - G_{44}, \\
 K_{45} = K_{54} &= -C_{11} \alpha^2 - C_{66} \beta^2 - I_{44}, \\
 K_{46} = K_{64} &= -A_{SS12} \alpha \beta - A_{SS166} \alpha \beta, \\
 K_{47} = K_{74} &= -C_{12} \alpha \beta - C_{66} \alpha \beta, \\
 K_{48} = K_{84} &= \left(L_{13} \alpha - G_{55} \alpha + \frac{Q_{511}}{R_1} + \frac{Q_{512}}{R_2} \right), \\
 K_{49} = K_{94} &= \left(L_{213} \alpha - I_{55} \alpha + \frac{Q_{611}}{R_1} + \frac{Q_{612}}{R_2} \right), \\
 K_{55} &= -A_{SS211} \alpha^2 - A_{SS266} \beta^2 - H_{44}, \\
 K_{56} = K_{65} &= -C_{12} \alpha \beta - C_{66} \alpha \beta, \\
 K_{57} = K_{75} &= -A_{S212} \alpha \beta - A_{SS266} \alpha \beta, \\
 K_{58} = K_{85} &= -I_{55} \alpha + M_{113} \alpha + \frac{Q_{711}}{R_1} + \frac{Q_{712}}{R_2}, \\
 K_{59} = K_{95} &= -H_{55} \alpha + M_{213} \alpha + \frac{Q_{811}}{R_1} + \frac{Q_{812}}{R_2}, \\
 K_{66} &= -A_{SS122} \beta^2 - A_{SS166} \alpha^2 - G_{55}, \\
 K_{67} = K_{76} &= -C_{22} \beta^2 - C_{66} \alpha^2 - I_{55}, \\
 K_{68} = K_{86} &= L_{123} \beta - G_{55} \beta + \frac{Q_{512}}{R_1} + \frac{Q_{522}}{R_2}, \\
 K_{69} = K_{96} &= L_{223} \beta - I_{44} \beta + \frac{Q_{612}}{R_1} + \frac{Q_{622}}{R_2}, \\
 K_{77} &= -A_{SS222} \beta^2 - A_{SS266} \alpha^2 - H_{55}, \\
 K_{78} = K_{87} &= M_{123} \beta - I_{44} \beta + \frac{Q_{712}}{R_1} + \frac{Q_{722}}{R_2}, \\
 K_{79} = K_{97} &= M_{223} \beta - H_{44} \beta + \frac{Q_{812}}{R_1} + \frac{Q_{822}}{R_2}, \\
 K_{88} &= -G_{55} \alpha^2 - G_{44} \beta^2 - N_{133} - 2 \frac{Q_{1613}}{R_1} \\
 &\quad - 2 \frac{Q_{1623}}{R_2} - \frac{Q_{1311}}{R_1^2} - 2 \frac{Q_{1312}}{R_1 R_2} - \frac{Q_{1322}}{R_2^2}, \\
 K_{89} = K_{98} &= -I_{55} \alpha^2 - I_{44} \beta^2 - N_{333} - \frac{Q_{1713}}{R_1} \\
 &\quad - \frac{Q_{1723}}{R_2} - \frac{Q_{1813}}{R_1} - \frac{Q_{1823}}{R_2} - \frac{Q_{1411}}{R_1^2} - 2 \frac{Q_{1412}}{R_1 R_2} - \frac{Q_{1422}}{R_2^2}, \\
 K_{99} &= -H_{55} \alpha^2 - H_{44} \beta^2 - N_{233} - 2 \frac{Q_{1913}}{R_1} \\
 &\quad - 2 \frac{Q_{1923}}{R_2} - \frac{Q_{1511}}{R_1^2} - 2 \frac{Q_{1512}}{R_1 R_2} - \frac{Q_{1522}}{R_2^2}
 \end{aligned}$$

The elements of mass matrix [M] in Eq. (16) are: [A.2]

$$\begin{aligned}
 M_{11} &= - \left(I_1 + 2 \frac{I_2}{R_1} + \frac{I_3}{R_1^2} \right), \\
 M_{13} &= - \alpha \left(I_2 + \frac{I_3}{R_1} \right), \\
 M_{14} &= - \left(I_4 + \frac{I_8}{R_1} \right), \\
 M_{15} &= - \left(I_5 + \frac{I_9}{R_1} \right), \\
 M_{22} &= - \left(I_1 + 2 \frac{I_2}{R_2} + \frac{I_3}{R_2^2} \right), \\
 M_{23} &= - \beta \left(I_2 + \frac{I_3}{R_2} \right), \\
 M_{26} &= - \left(I_4 + \frac{I_8}{R_2} \right), \\
 M_{27} &= - \left(I_5 + \frac{I_9}{R_2} \right), \\
 M_{33} &= - (I_3 \alpha^2 + I_3 \beta^2 + I_1), \\
 M_{34} = I_8 \alpha, \quad M_{35} = I_9 \alpha, \\
 M_{36} = I_8 \beta, \quad M_{37} = I_9 \beta, \\
 M_{38} = -I_{10}, \quad M_{35} = -I_{11}, \\
 M_{44} = -I_6, \quad M_{45} = -I_{15}, \\
 M_{55} = -I_7, \quad M_{66} = -I_6, \\
 M_{67} = -I_{15}, \quad M_{77} = -I_7, \quad M_{88} = -I_{13}, \\
 M_{89} = -I_{12}, \quad M_{99} = -I_{14} \\
 M_{12} = M_{16} = M_{17} = M_{18} = 0 \\
 M_{19} = M_{24} = M_{25} = M_{28} = M_{29} = 0 \\
 M_{46} = M_{47} = M_{48} = M_{49} = 0 \\
 M_{56} = M_{57} = M_{58} = M_{59} = M_{68} = 0 \\
 M_{69} = M_{78} = M_{79} = 0
 \end{aligned}$$

$\Delta = \{u_1, \phi_{x1}, \psi_{x1}, v_1, \phi_{y1}, \psi_{y1}, w_1, \phi_{z1}, \psi_{z1}\}^T$
 where [A.3] is,

$$\begin{aligned}
 (A_{ij}, B_{ij}, D_{ij}) &= C_{ij} \int_{-h/2}^{h/2} [1.0, z, z^2] dz \\
 (A_{S1j}, A_{S2j}) &= C_{ij} \int_{-h/2}^{h/2} [\varphi_1(z), \varphi_2(z)] dz
 \end{aligned}$$

$$\begin{aligned} (B_{S1_{ij}}, B_{S2_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} [z \varphi_1(z), z \varphi_2(z)] dz \\ (Q_{1_{ij}}, Q_{2_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} [\varphi_1(z), \varphi_2(z)] dz \\ (Q_{3_{ij}}, Q_{4_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} [z \dot{\varphi}_1(z), z \dot{\varphi}_2(z)] dz \\ (A_{SS1_{ij}}, A_{SS2_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} \{[\varphi_1(z)]^2, [\varphi_2(z)]^2\} dz \\ (C_{ij}) &= C_{ij} \int_{-h/2}^{h/2} \{[\varphi_1(z) \varphi_2(z)]\} dz \\ (Q_{5_{ij}}, Q_{6_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} \{\varphi_1(z) [\dot{\varphi}_1(z), \dot{\varphi}_2(z)]\} dz \\ (Q_{7_{ij}}, Q_{8_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} \{\varphi_2(z) [\dot{\varphi}_1(z), \dot{\varphi}_2(z)]\} dz \\ (Q_{13_{ij}}, Q_{15_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} \{[\dot{\varphi}_1(z)]^2, [\dot{\varphi}_2(z)]^2\} dz \\ (Q_{14_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} \{[\dot{\varphi}_1(z) \dot{\varphi}_2(z)]\} dz \\ (G_{ij}, H_{ij}) &= C_{ij} \int_{-h/2}^{h/2} \{[\dot{\varphi}_1(z)]^2, [\dot{\varphi}_2(z)]^2\} dz \\ (I_{ij}) &= C_{ij} \int_{-h/2}^{h/2} \{[\dot{\varphi}_1(z) \dot{\varphi}_2(z)]\} dz \\ (E_{ij}, F_{ij}) &= C_{ij} \int_{-h/2}^{h/2} \{[\varphi_1''(z)], [\varphi_2''(z)]\} dz \\ (N_{1_{ij}}, N_{2_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} \{[\varphi_1''(z)]^2, [\varphi_2''(z)]^2\} dz \\ (N_{3_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} \{[\varphi_1''(z) \varphi_2''(z)]\} dz \\ (Q_{17_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} \{\dot{\varphi}_1(z) \varphi_1''(z)\} dz \\ (Q_{17_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} \{\dot{\varphi}_1(z) \varphi_2''(z)\} dz \\ (Q_{18_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} \{\varphi_2(z) \varphi_1''(z)\} dz \\ (Q_{19_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} \{\varphi_2(z) \varphi_2''(z)\} dz \\ (J_{ij}) &= C_{ij} \int_{-h/2}^{h/2} z \varphi_1''(z) dz \\ (L_{1_{ij}}, L_{2_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} \varphi_1''(z) [\varphi_1(z), \varphi_2(z)] dz \\ (O_{ij}) &= C_{ij} \int_{-h/2}^{h/2} z \varphi_2''(z) dz \\ (M_{1_{ij}}, M_{2_{ij}}) &= C_{ij} \int_{-h/2}^{h/2} \varphi_2''(z) [\varphi_1(z), \varphi_2(z)] dz \end{aligned}$$

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